

Predicting channel bed topography in hydraulic fall experiments

Alexander Tam Zheng Yu Richard Kelso Ben Binder

School of Mathematical Sciences and School of Mechanical Engineering
The University of Adelaide

February 10, 2016



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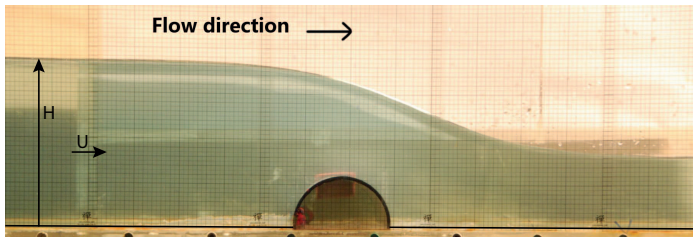


Hydraulic fall

- Waveless flow upstream and downstream
- Characterised by upstream Froude number

$$F = \frac{U}{\sqrt{gH}} = \frac{Q}{WH\sqrt{gH}} \quad (\text{flow rate } Q = WHU)$$

- Weakly nonlinear and fully nonlinear models
- Forward and inverse methods



Experimental method

- Closed-loop water channel
- Pump frequency controls flow rate, Froude number

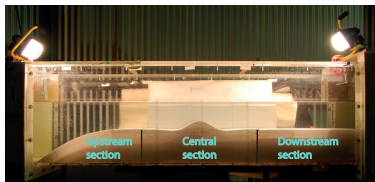
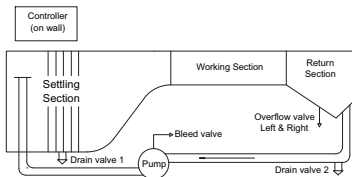
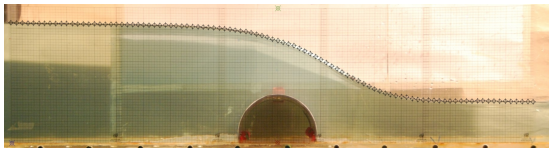
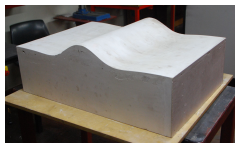
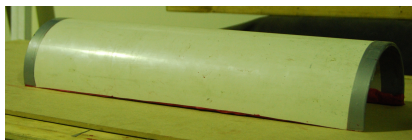


Figure : Water channel

Experimental method

- Semicircular and Gaussian topography
- Nikon D40X used to photograph steady hydraulic falls
- Spotlights, grids, dye enhance visualisation
- Free surface profiles (\hat{x}_j, \hat{y}_j) extracted from images



- Steady flow
- Irrotational flow: $\mathbf{v} = (u, v) = \nabla\phi$
- Incompressible fluid: $\nabla \cdot \mathbf{v} = 0$

Dimensionless model equations:

$$\phi_{xx} + \phi_{yy} = 0 \quad \text{for } \sigma < y < 1 + \eta$$

$$\phi_y = \phi_x \eta_x \quad \text{on } y = 1 + \eta$$

$$\frac{1}{2}(\phi_x^2 + \phi_y^2) + \frac{1}{F^2}y = \frac{1}{2} + \frac{1}{F^2} \quad \text{on } y = 1 + \eta$$

$$\phi_y = \phi_x \sigma_x \quad \text{on } y = \sigma$$

$$\phi \rightarrow x \quad \text{and} \quad y \rightarrow 1 \quad \text{as} \quad x \rightarrow -\infty$$

- Forced Korteweg-de Vries (KdV) equation

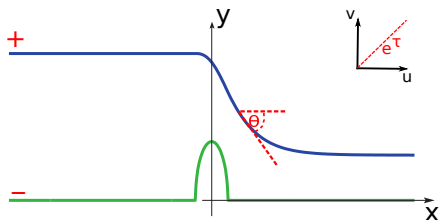
$$\eta_{xx} + \frac{9}{2}\eta^2 - 6(F - 1)\eta = -3\sigma$$

Forward problem – unknown F and η

$$\frac{d^2\eta}{dx^2} + \frac{9}{2}\eta^2 - 6(F - 1)\eta = -3\sigma_E$$

Inverse Problem – unknown σ

$$\sigma = 2(F_E - 1)\eta_E - \frac{1}{3} \frac{d^2\eta_E}{dx^2} - \frac{3}{2}\eta_E^2$$



- Model based on Binder, Blyth, and McCue (2013)

$$e^{2\tau^+} + \frac{2}{F^2}y^+ = 1 + \frac{2}{F^2}, \quad \tau = \log |\mathbf{v}|$$

$$\tau^\pm(s) = \int_{-\infty}^{\infty} \frac{\theta^-(t)}{1 \pm e^{\pi(s-t)}} - \frac{\theta^+(t)}{1 \mp e^{\pi(s-t)}} dt$$

$$x^\pm(s) = x^\pm(-\infty) + \int_s^{\infty} e^{-\tau^\pm(t)} \cos \theta^\pm(t) dt$$

$$y^\pm(s) = y^\pm(-\infty) + \int_s^{\infty} e^{-\tau^\pm(t)} \sin \theta^\pm(t) dt$$

Semicircle results

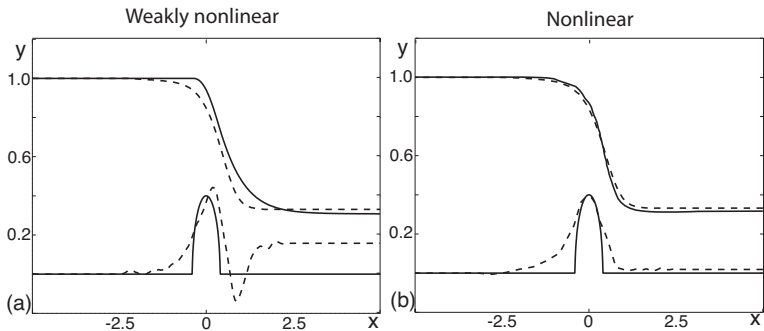


Figure : Solid curves: forward problem. Broken curves: inverse problem. $F_E = 0.38$. (Tam et al. 2015)

- Forward nonlinear solution from Forbes (1988)

Semicircle results

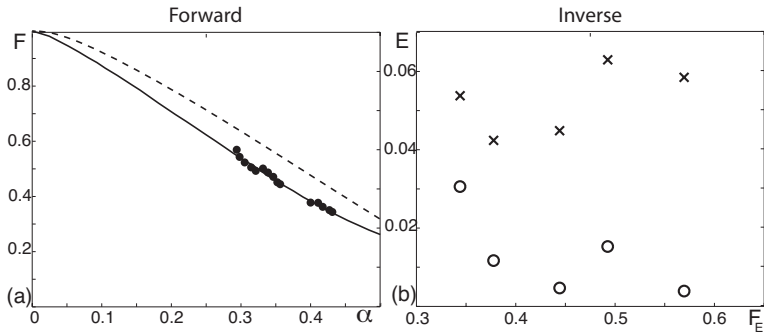


Figure : (a) Nonlinear (solid), weakly nonlinear (broken), and experiment (markers). (b) $E = |M - \alpha|$, Nonlinear (circles), weakly nonlinear (crosses)

- Weakly nonlinear model over-estimates Froude number
- Both models accurately predict maximum topography height

Gaussian results

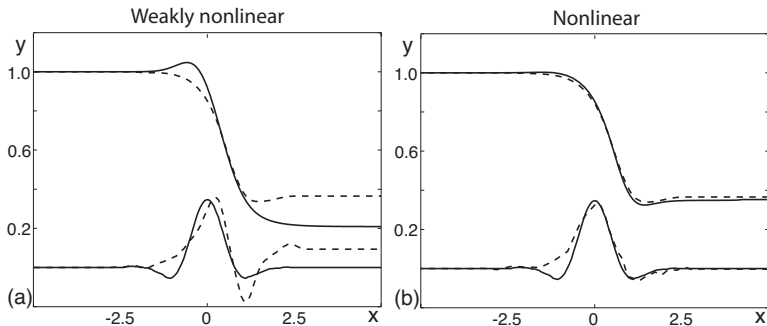





Figure : Solid curves: forward problem. Broken curves: inverse problem.
 $F_E = 0.45$.

- Smooth topography reduces error at bump supports
- Recirculatory regions not predicted by potential flow model

- Weakly nonlinear (KdV) and nonlinear models
- Inverse methods used to predict topography in experiments
- Both models accurately predict maximum topography height
- Nonlinear model accurately predicts topography shape

-  Binder, B.J., Blyth, M.G., and McCue, S.W. 2013, “Free-surface flow past arbitrary topography and an inverse approach for wave-free solutions”, *IMA Journal of Applied Mathematics*, vol. 78, no. 4, pp. 685–696.
-  Forbes, L.K. 1988, “Critical free-surface flow over a semi-circular obstruction”, *Journal of Engineering Mathematics*, vol. 22, no. 1, pp. 3–13.
-  Tam, A., Yu, Z., Kelso, R.M., and Binder, B.J. 2015, “Predicting channel bed topography in hydraulic falls”, *Physics of Fluids*, vol. 27(112106),